

AEROBIC DIGESTION Not the Same Old Same Old

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Aerobic Digestion

Not Treatment of Raw Waste

Endogenous Respiration

All Bugs + No Food = Cannibalism

Reduces Volatile Solids

Kills Pathogenic Organisms



Aerobic Digestion

Pros and Cons

Advantages	Disadvantages
Lower capital costs for facilities with capacities less than 19,000 m ³ /day (5 MGD)	Higher operating costs and other operational issues when treating primary sludge
Minimal nuisance odors (except for short SRT ATAD processes)	Higher energy costs than other stabilization processes, especially traditional mesophilic anaerobic digestion
Simple construction	Limited pathogen reduction (except for ATAD)
No danger of explosions or suffocations	Lower cake solids (except for some ATAD processes)
Simple operation	Potential for alkalinity depletion if nitrification occurs
Weaker sidestreams	
Less impact from low pH	

What is the End Use?

Marketing/Distribution?

- Class A Pathogen
 Reduction
- EQ Class Requirements

Public Access?

Class A Pathogen
 Reduction

Landfill?

Vector Attraction Reduction Criteria

Restricted Access?

Class B Pathogen
 Reduction

Aerobic Digestion and Pathogen Reduction

- Class A Pathogen Reduction
 - PFRP Processes
 - Thermophilic Aerobic Digestion
 - PFRP-Equivalent Processes
 - ATAD Autothermal Thermophilic Aerobic Digestion
 - Alternative 1 Time/Temperature (Thermophilic Systems)
- Class B Pathogen Reduction
 - PSRP Specifically Defined Operating Criteria (40 CFR 257)
 - Aerobic Digestion
 - Alternative 1 Testing

What's Not Listed?

Unless Defined Criteria Are Met, Digestion Processes Are Not PFRP or PSRP

For PFRP

May Meet Alternative 1, if Process is Thermophilic

May Meet Alternative 4 (Testing)

Less Desirable

Alternative Processes – NOT LISTED Thickened Aerobic Digestion Modifications Aerobic/Anoxic Digestion Aerobic/Anaerobic Digestion Most Proprietary Processes



Autothermal thermophilic aerobic digestion (ATAD) is listed as a PFRP process.

True False



All solids treated with a PFRP can be reused through marketing and distribution.

	True	
	False]
·		-

Aerobic Digestion and Vector Attraction Reduction

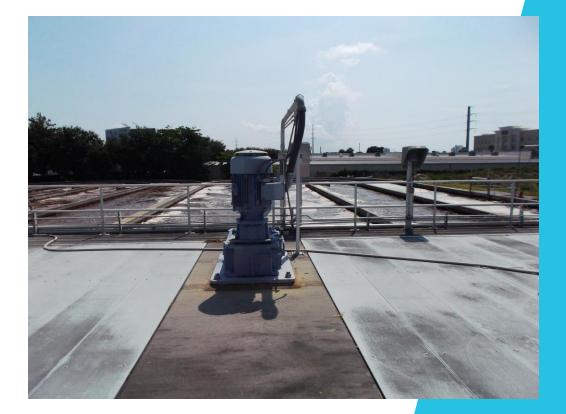
Option 1 - Volatile Solids Reduction (38%)

Measured From Entry to Solids Process

Up to 50% - Dependent on Prior Processes

More Difficult with Extended Aeration Processes

20-25% Of Influent Solids Are Refractory



Calculating Volatile Solids Reduction

Not (Mass In – Mass Out)/Mass In

Appropriate Method Varies

Depends Upon

Grit Accumulation

Decantate Removal

Appropriate Methods

	Full Mass Balance	Approx. Mass Balance	Van Kleeck
No Decantate/ No Grit Accumulation	\checkmark	\checkmark	\checkmark
Grit Accumulation	\checkmark	\checkmark	
Decantate Withdrawal	\checkmark	\checkmark	\checkmark
Decantate Withdrawal and Grit Accumulation	\checkmark	\checkmark	

Van Kleeck Equation

$$FVSR = \frac{VS_f - VS_b}{VS_f - (VS_f \times VS_b)}$$

Where:

FVSR = Fractional Volatile Solids Reduction

VS_f = Feed Sludge Fractional Volatile Solids (kg/kg)

VS_b = Digested Sludge Fractional Volatile Solids (kg/kg)

Aerobic Digestion and Vector Attraction Reduction

Other Available Options:

Option 3 – Additional Digestion of Aerobically Digested Sewage Sludge Limited to Sludges with 2% or less Solids Thicker Sludges Can Be Diluted Less than 15% Additional Reduction after Additional 30 Days Option 4 – Specific Oxygen Uptake Rate (SOUR) Limited to Sludges with 2% or less Solids Dilution is not allowed Sludges from 10 - 30° C ($50 - 86^{\circ}$ F) During Summer, Sludge May Be Warmer Not Suitable for Thermophilic Systems Holding Time Less Than 2 hours



An aerobic digester is operated with good mixing and bottom withdrawal of biosolids (no grit accumulation). Telescoping valves are used to periodically decant the digester. Volatile Solids Reduction can be calculated using

- a. The Van Kleeck Equation
- b. Approximate Mass Balance
- _ c. Full Mass Balance
 - d. All of the Above
 - e. Can not be solved



A treatment plant utilizes aerobic digestion, with typical summer temperatures (both air and liquid) over 85 degrees F. The sludge is thickened to 2.5% solids in the digester. Vector attraction reduction criteria can be demonstrated using:

(Select all that apply)

- a. Option 1 Volatile Solids Reduction
- b. Option 3 Additional Aerobic Digestion
- c. Option 4 SOUR test

Aerobic Digestion (Conventional)

Class B Process – Defined as PSRP

PSRP Operational Criteria

40 Days at 20 Degrees C

60 Days at 15 Degrees C

No Allowance for Temperatures Outside This Range

Detention Time Credit for Two-Stage (True Series) Operation

Referenced in EPA Documents, Up to State Regulatory Agency

30% Reduction in Detention Time (Required Volume)

Also valid for true Batch Operation (Draw, then Fill)



Aerobic Digestion Fundamentals

Based on Endogenous Respiration

- $C_5H_7O_2N + 5O_2 => 4CO_2 + H_2O + NH_4HCO_3$
- Produces both Ammonia and Alkalinity

If Sufficient Oxygen is Provided

NH₄ + 2O₂ => NO₃⁻ + 2H⁺ + H₂O
Consumes Alkalinity

If Alkalinity drops low enough, only partial nitrification

• $2C_5H_7O_2N + 12O_2 => 10CO_2 + 5H_2O + NH_4^+ + NO_3^-$

Calculating Detention Time

Two Factors:

Regulatory Requirements - Class B Process

Biological Requirements

See Handout





Aerobic digestion consumes alkalinity.

- a. True
- b. False
- c. Depends upon the extent of aeration



At 20 degrees C, if an aerobic digester is operated in series, the minimum allowable detention time is:

- a. 40 days
- b. 42 days
- c. 60 days

d.

none of the above

Oxygen Requirements

Theoretical: $1.5/2.0 \text{ kg O}_2/\text{kg}$ active cell mass

Field: 2.0 kg O_2 /kg active cell mass

+ 1.6 to 1.9 kg O₂/kg VS destroyed for Primary Sludge



Oxygen Transfer/Mixing Requirements

At 1 to 2% Solids, mixing will typically govern

Mixing Requirements:

10 to 100 W/m³ (0.5 to 4.0 hp/1000 cu ft)

0.33 to 0.50 L/m³•s (20 to 30 cu ft/min/1000 cu ft)

Typical Oxygen transfer requirements (not thickened)

WAS ONLY - 0.25 to 0.33 L/m³•s (15 to 20 cu ft/min/1000 cu ft)

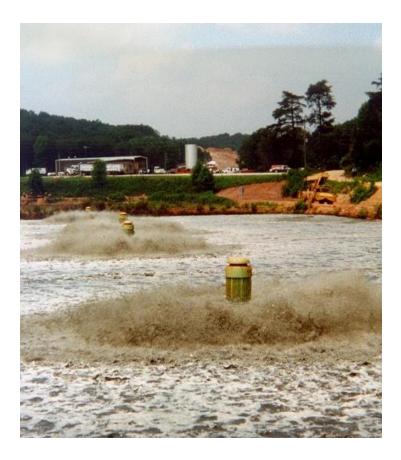
PRIMARY AND WAS - 0.40 to 0.50 L/m³ s (25 to 30 cu ft/min/1000 cu ft)

DO Typically Maintained at 2.0 mg/L, unless OUR < 20 mg/L•h

Aeration Methods

Mechanical Aerators Coarse Bubble Fine Bubble Jet Aeration Other

Mechanical Aerators

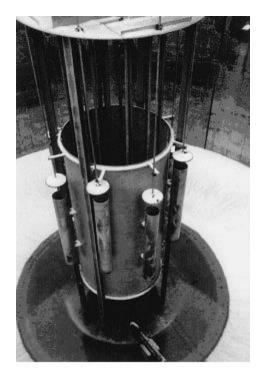




Photos courtesy Evoqua Water Technologies

Coarse Bubble Aeration









Photos courtesy Evoqua Water Technologies

Fine Bubble Aeration





Photos courtesy Evoqua Water Technologies

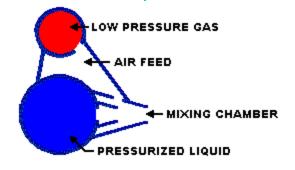
Jet Aeration













For digesters without primary sludge, 20 to 30 SCFM/1000 ft³ will typically provide sufficient energy for mixing and sufficient oxygen for aeration.





The typical oxygen requirement for aerobic digesters (without primary sludge) is:

- a. 2.0 kg O₂/kg VS
 b. 10 SCFM/1000 ft³
- c. none of the above

Instrumentation & Controls

Not Mandatory, But Helpful

Typical Controls:

Dissolved oxygen

ORP – can be beneficial either for anoxic/aerobic systems or thermophilic systems

Tank level control



Sidestream Characteristics

Parameter	Typical Range	Acceptable Value
рН	5.9-7.7	7.0
5-day BOD (mg/L)	9-1700	500
Filtered 5-day BOD (mg/L)	4-173	50
Suspended Solids (mg/L)	46-2000	1000
Kjehldahl Nitrogen (mg/L)	10-400	170
Nitrate-nitrogen (mg/L)	0-30	10
Total Phosphorus (mg/L)	19-241	100
Soluble Phosphorus (mg/L)	2.5-64	25

Controlling Nutrients

Nitrogen Control

Aerobic/Anoxic Operation

Phosphorus Control

Controlling Phosphorus to Disposal

Controlling Phosphorus in Sidestream

Aerobic/Anoxic Operation

Aeration cycling promotes denitrification

Lower DO to less than 1 mg/L (maintain mixing)

ORP Control beneficial

Reduces energy demand (up to 18%)

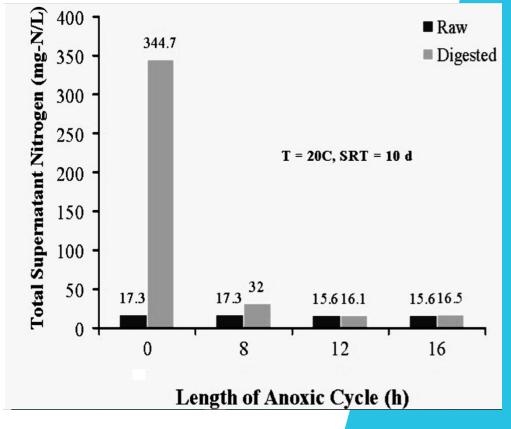
Alkalinity credit (up to 50%)

$$C_5H_7O_2N + 4NO_3^- + H_2O => NH_4^+ + 5HCO_3^- + 2N_2^{\uparrow}$$

Equations Combine

 $2C_5H_7O_2N + 11.5O_2 => 10CO_2 + N_2 + 7H_2O$

Nitrogen goes to atmosphere – not sludge or recycle...



From Al-Ghusain et al, 2004

Phosphorus Control

Not Destroyed

Either To Disposal or To Effluent

Release in Aerobic Digesters is Less

Limiting Sidestream P Concentrations

Aerobic/Anoxic Operation (~50% Reduction vs Continuous Aeration)

pH Control

pH <6 increases release

Lime addition reduces release

Partial Nitrification => Formation/Removal of Struvite

Keys to Design

Take Advantage of the Series "Credit" – 30% Reduction in Volume

At least two tanks

Draw, then fill

Provide Enough Air

Fine bubble aeration is possible

Energy savings, but higher maintenance

Ability to re-suspend solids, or provide mixers + aeration

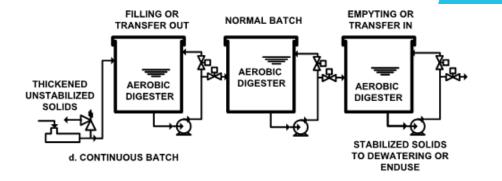
Allows anoxic/aerobic cycling and denitrification

Allow for primary sludge requirements

Thickening decreases aeration efficiency (>~3%solids)

Utilize in-tank thickening where feasible

Further reduces tank volume



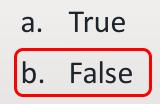


Pick which of the following statements is true.

- a. Anoxic/aerobic digestion will increase the supernatant phosphorus concentration.
- b. Anoxic/aerobic digestion will decrease the supernatant nitrogen concentration.
- c. Anoxic/aerobic digestion will increase supernatant phosphorus and nitrogen concentrations.



Turning the air off in an aerobic digester for up to 8 hours will have an adverse impact on the digester supernatant.



Design Techniques to Optimize Digestion

Do Not Result in PFRP or PFRP-equivalency

May Meet Class A Using Other Alternatives

May Reduce Capital Costs

Thickening

Decreases volume required for given HRT

Negatively impacts aeration efficiency

Multiple Variants – Pre-thickening, In-loop, Post-thickening

Aerobic/Anoxic Operation

Digester Decanting

Can be Batch Operation or Decanting

Can Achieve Up to 2.5% Solids

Advantages

No additional basins are required

Possible to use existing tanks to both thicken and digest

Disadvantages

Larger basins required (low solids concentration prior to decanting)

Varying liquid levels may impact aeration efficiency

No control of alkalinity, temperature, nitrogen or phosphorus



In-Loop (Recuperative) Thickening

Two Main Phases

In-loop

Isolation

Four Main Basins

Two digesters, One premix, one thickener

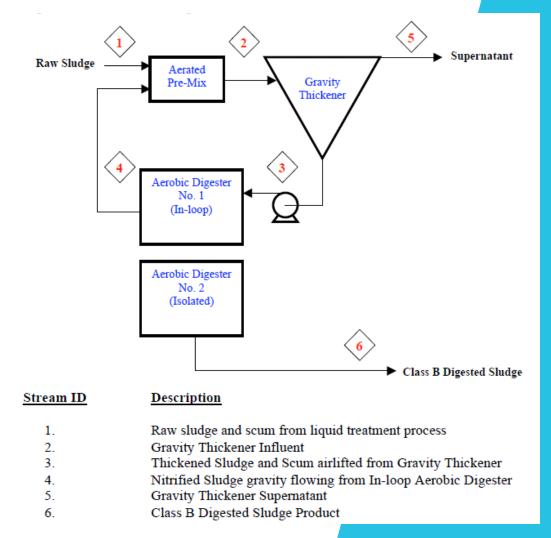
Advantages

Provides benefits of anoxic/aerobic operation

Controls nitrogen and phosphorus in supernatant

Disadvantages

Higher capital cost



Membrane Thickening

Membranes can be mounted in digester basin or in separate basin

Operated in batch or continuous mode

Advantages

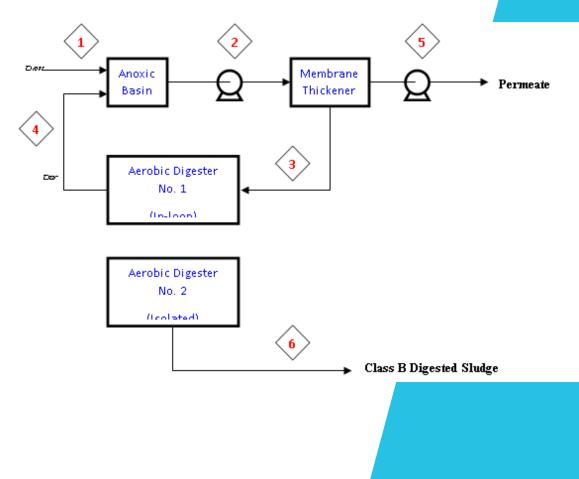
Physical barrier of membrane provide best control of supernatant quality

Small footprint

Control of solids concentration

Disadvantages

Capital cost



Post-thickening

Resembles Activated Sludge Process – Separation/Thickening Downstream of Digestion

Advantages

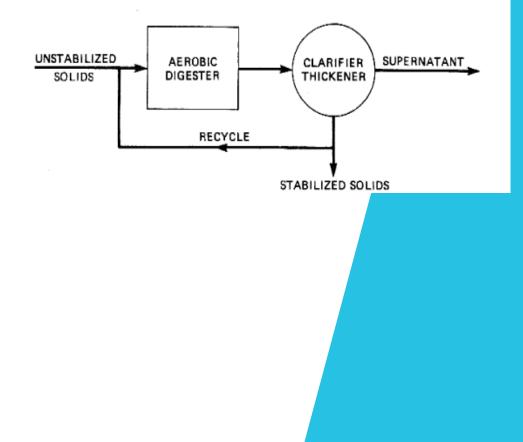
Digester operates at fixed level – overflow goes to separator

Disadvantages

Digesters sized based on lower solids concentration

Higher O&M costs

No control of alkalinity, temperature, nitrogen or phosphorus



Impact of Solids on Aeration

Alpha Factors vs MLSS

From MBR Research

Coarse Bubble α varies widely

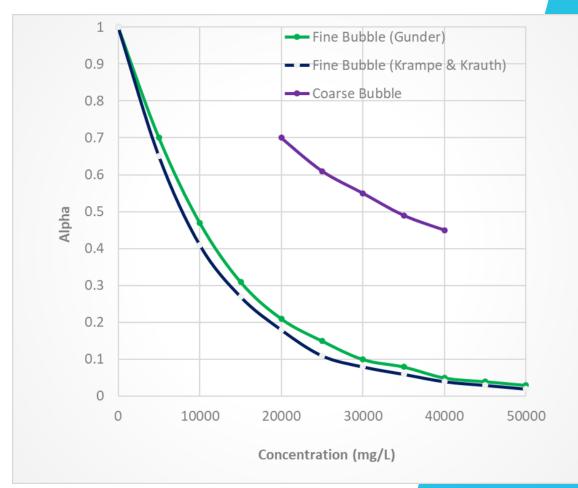
Data from membrane tank

Focus on trend, not #

Verify w/ manufacturer

Dramatic decrease

Due to viscosity



Source: Final Report, WERF Project #00-CTS-8a, MBR Website Strategic Research, Relationship between MLSS and Oxygen Transfer Efficiency in MBR Systems



Alpha factors:

- 1. Increase with solids concentration for both coarse and fine bubble diffusers
- 2. Decrease with solids concentration for both coarse and fine bubble diffusers
- 3. Increase with solids concentration for coarse bubble diffusers but decrease with concentration for fine bubble diffusers.



Which of the following methods is NOT a way to reduce aerobic digester volumes required to meet the SRT requirements for PSRP?

- 1. Decant the digester to remove supernatant
- 2. Provide in-loop membrane thickening
- 3. Dewater digested solids using centrifuges
- 4. Utilize two stages in series



Thank You!

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